PE Civil Structural Practice Exam

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Errata posted 8-16-2017

Revisions are shown in red.

Question 521, p. 47:

Question 521 should read as follows:

521. The W10 \times 22 steel beam (F_V = 50 ksi) shown in the figure is only braced at the center of span.

Work either the ASD or the LRFD version of the question, considering the most conservative instance.

<u>ASD</u>

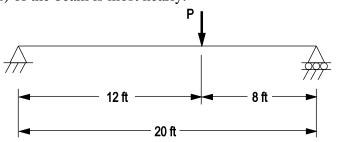
The allowable flexural strength (ft-kips) of the beam is most nearly:

- (A) 45
- (B) 51
- (C) 56
- (D) 65

LRFD

The design moment capacity ϕ Mn (ft-kips) of the beam is most nearly:

- (A) 68
- (B) 76
- (C) 84
- (D) 97



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Solution 521, p. 96:

Solution 521 should read as follows:

521. The properties of a W10 \times 22 are as follows:

AISC, 14th ed., Table 3-2.

$$\begin{split} &\phi_b M_p x = 97.5 \text{ ft-kips} \\ &M_p x/\Omega_b = 64.9 \text{ ft-kips} \\ &BF/\Omega_b = 2.68 \text{ kips (ASD) or } \phi_b BF = 4.02 \text{ kips (LRFD)} \\ &L_p = 4.7 \text{ ft} \end{split}$$

$$L_p = 4.710$$

$$L_b = 10 \text{ ft}$$

ASD:
$$M_p x/\Omega_b - BF(L_b - L_p) = M_n/\Omega_b$$

$$64.9 - 2.68(10 - 4.7) = 50.7$$
 ft-kips

LRFD:
$$\phi_b M_p x - BF(L_b - L_p) = \phi_b M_n$$

 $97.5 - 4.02(10 - 4.7) = 76.2 \text{ ft-kips}$

THE CORRECT ANSWER IS: (B)

Previously posted errata continued on next page

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Solution 533, p. 96:

Line 4 should read as follows:

Allowable load = 2(13)(0.79) = 21.2 kips

Previously posted errata continued on next page

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Revisions are shown in red.

Question 509, p. 38:

Question 509 should read as follows:

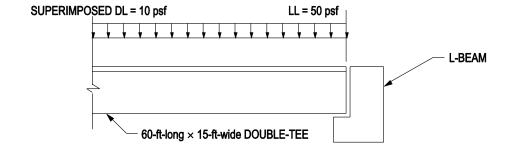
A double-tee supported by an L-beam is loaded as shown in the figure.

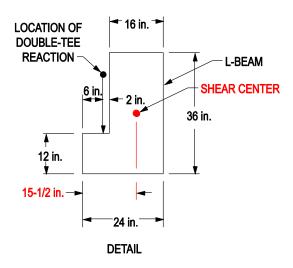
Design Code:

ACI 318: Building Code Requirements for Structural Concrete, 2011.

The magnitude of the ultimate torsion (ft-kips) induced into the L-beam, by superimposed dead and live load applied on the double-tee, is most nearly:

- (A) 18.7
- (B) 21.4
- (C) 28.6
- (D) 32.8





NOT TO SCALE

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Errata posted 8-25-2016

Solution 509, p. 87:

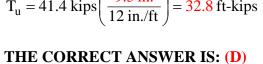
Solution 509 should read as follows:

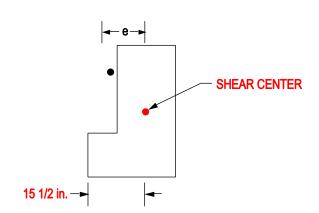
$$15.5 \text{ in.} - 6 \text{ in.} = 9.5 \text{ in.}$$

$$W_{u/DT} = [1.2(10 \text{ psf}) + 1.6(50 \text{ psf})]15 \text{ ft} = 1,380 \text{ plf}$$

$$R_{u/DT} = \frac{1}{2} (1.38 \text{ klf})(60 \text{ ft}) = 41.4 \text{ kips}$$

$$T_u = 41.4 \text{ kips} \left(\frac{9.5 \text{ in.}}{12 \text{ in./ft}} \right) = 32.8 \text{ ft-kips}$$





Question 513, p. 41:

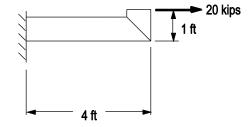
Question 513 should read as follows:

The pipe member shown in the figure has a constant section and the following properties:

Outside diameter	10 in.
Moment of inertia	90 in ⁴
Area	7 in^2

Neglecting the weight of the pipe, the maximum compressive stress (ksi) at the support is most nearly:

- (A) 2.9
- (B) 4.9
- (C) 13.3
- 16.2 (D)



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Solution 513, p. 88:

Solution 513 should read as follows:

$$\begin{split} &M_S = \left(20 \text{ kips}\right) \left(12 \text{ in.} - 5 \text{ in.}\right) = 140 \text{ in.-kips} \\ &N_S = 20 \text{ kips} \\ &\sigma_{flexure} = \frac{(140 \text{ in.-kips})(5 \text{ in.})}{90 \text{ in}^4} = 7.8 \text{ ksi } \begin{cases} \text{Top tension} \\ \text{Bottom compression} \end{cases} \\ &\sigma_{axial} = \frac{20 \text{ kips}}{7 \text{ in}^2} = 2.9 \text{ ksi tension} \end{split}$$

THE CORRECT ANSWER IS: (B)

Solution 520, p. 91:

Line 2 should read as follows:

-7.8 ksi + 2.9 ksi = -4.9 ksi

Two or more spans not exceeding 10 ft

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Question 528, p. 52:

Question 528 should read as follows:

A connection is shown in the figure.

Design Code:

AISC: Steel Construction Manual, 14th edition.

Material:

A36 steel

Assumptions:

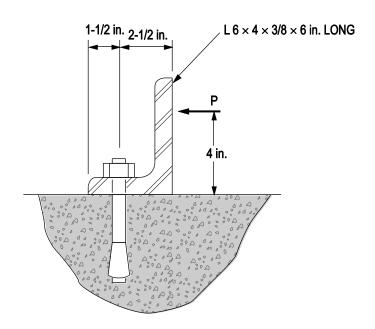
The bolt is sufficient.

The concrete is sufficient.

The load P is equally distributed along the entire 6-in.-long angle.

The maximum load P (ASD) or ϕP_n (LRFD) in kips that can be carried by the angle is most nearly:

	ASD	LRFD
(A)	0.8	1.2
(B)	1.1	1.7
(C)	1.2	1.8
(D)	2.0	2.0



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Solution 528, p. 94:

Solution 528 should read as follows:

By inspection P controls.

$$\begin{split} M_n &= M_p = F_y Z_x \leq 1.6 \ M_y \\ F_y &= 36 \ ksi \\ S_x &= 1/6 \ bd^2 = 1/6 \ (6)(0.375)^2 = 0.141 \ in^3 \\ M_y &= F_y S_x = (36)(0.141) = 5.076 \ in.\text{-kips} \\ Z_x &= 1/4 \ bd^2 = 1/4 \ (6)(0.375)^2 = 0.211 \ in^3 \\ M_p &= F_y Z_x = (36)(0.211) = 7.6 \ in.\text{-kips} & \leftarrow \text{governs} \\ \text{Check } M_p \leq 1.6 \ M_y & 1.6 \ (5.076) = 8.1 > 7.6 \ \therefore \ \text{OK} \end{split}$$

AISC ASD:

$$M_n/\Omega = 7.6/1.67 = 4.55 \text{ in.-kips}$$

$$P_{\text{allow}} = \frac{4.55}{\left[4 - (1/2)(3/8)\right]} = 1.19 \sim 1.2$$

AISC LRFD:

$$\phi M_n = 0.9(7.6) = 6.84 \text{ in.-kips}$$

$$\phi P_n = \frac{6.84}{\left\lceil 4 - \left(1/2\right)\left(3/8\right) \right\rceil} = 1.79 \sim 1.8$$

THE CORRECT ANSWER IS: (C)

Previously posted errata continued on next page

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Revisions are shown in red.

Solution 538, p. 98:

Solution 538 should read as follows:

Reference: AASHTO Section 9.7.2.3.

AASHTO prescriptively calls out to add back a single flange overhang (not one on each side).

$$L_{eff} = 8.5 \text{ ft} - b_f + \text{flange overhang}$$

= 8.5 ft - 1.5 ft + 0.25 ft
= 7.25 ft (7 ft 3in.)

THE CORRECT ANSWER IS: (C)