

**ERRATA for**  
**PE Civil Structural Practice Exam**  
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Revisions are shown in red.

**Question 61, p. 44**

The W10×22 steel beam ( $F_y = 50$  ksi) shown in the figure is braced at unknown intervals.

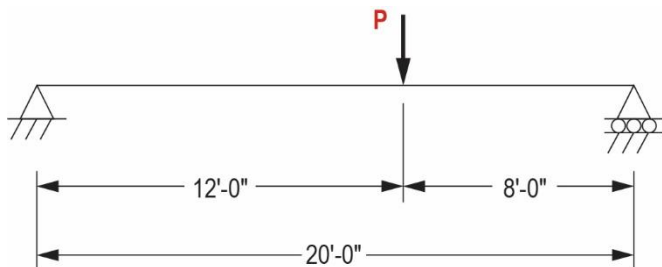
Work either the ASD or the LRFD version of the question. Assume  $c_b = 1.0$ .

**ASD**

The allowable flexural strength (ft-kips) of the beam is most nearly:

**LRFD**

The design moment capacity  $\phi M_n$  (ft-kips) of the beam is most nearly:

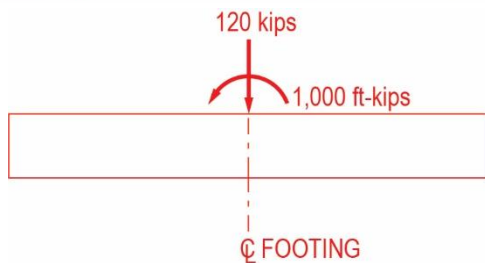


**Solution 70, p. 92**

Weight of footing =  $2.5 \text{ kips/ft} (24 \text{ ft}) = 60 \text{ kips}$

$$+\uparrow \sum F_v = -80 \text{ kips} - 60 \text{ kips} + 20 \text{ kips} = -120 \text{ kips}$$

$$+\curvearrowright \sum M_{\mathcal{C}} = -80 \text{ kips} (10 \text{ ft}) - 20 \text{ kips} (10 \text{ ft}) = -1,000 \text{ ft-kips}$$



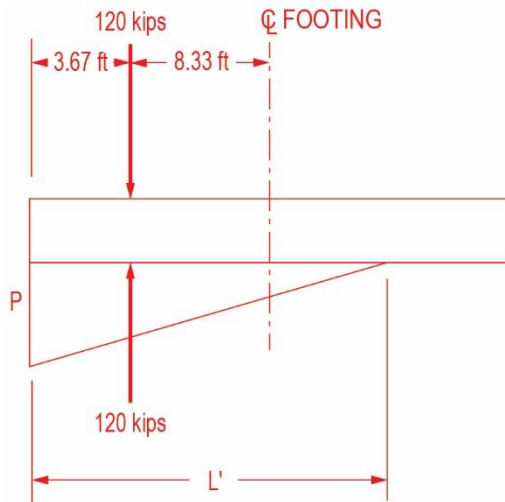
$$e = \frac{1,000 \text{ ft-kips}}{120 \text{ kips}}$$

$$e = 8.33 \text{ ft from } \mathcal{C}$$

$$\frac{L}{6} = \frac{24}{6} = 4 \text{ ft} < 8.33 \text{ ft}$$

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Resultant not within kern  
∴ Footing not fully effective



Resultant of soil pressure must align with eccentric resultant load.  
L' is the effective length of the triangular soil pressure pattern.

$$\therefore 3.67 \text{ ft} = \frac{L'}{3} \rightarrow L' = 3(3.67 \text{ ft}) = 11.01 \text{ ft}$$

$$\frac{1}{2} PL' = \frac{1}{2} P(11.01 \text{ ft}) = 120 \text{ kips} \rightarrow P = \frac{2(120 \text{ kips})}{11.01 \text{ ft}} = 21.8 \text{ kips/ft}$$

$$P_{\max} = \frac{P}{8 \text{ ft}} = \frac{21.8 \text{ kips/ft}}{8 \text{ ft}} = 2.72 \text{ kips/ft}^2$$

**THE CORRECT ANSWER IS: C**